**Truss Bridge System Optimization**

**EGR 7040 *Design Optimization*, Wright State University**

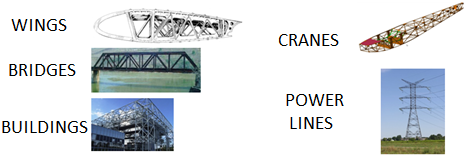
Joseph Strzelecki and Admir Makas

***Abstract***

*This report describes the process implemented to efficiently design a truss bridge for use by common foot and vehicular traffic. Design optimization techniques are used to formulate the design problem, generate a solution, and evaluate the effectiveness of the implemented solution. The project goal is to design a bridge spanning a 9 meter (~30 foot) ravine that must sustain external loads while minimizing bridge mass. By minimizing a mass cost function, monetary and material cost reductions can be achieved. The minimum mass of the bridge is constrained by the stress that each truss element of the bridge must withstand. As such an optimum design can be created for minimizing bridge mass subject to the proposed constraints. In order to calculate bridge stresses, finite element analysis (FEA) was implemented in conjunction with the optimization function FMINCON available in MATLAB. This technique was successful in generating an optimum design of the bridge system for different types of steel available in the market. Analysis shows that choosing the lightest design option does not necessarily yield the cheapest design. This result stems from the cost variability between different steel types.*

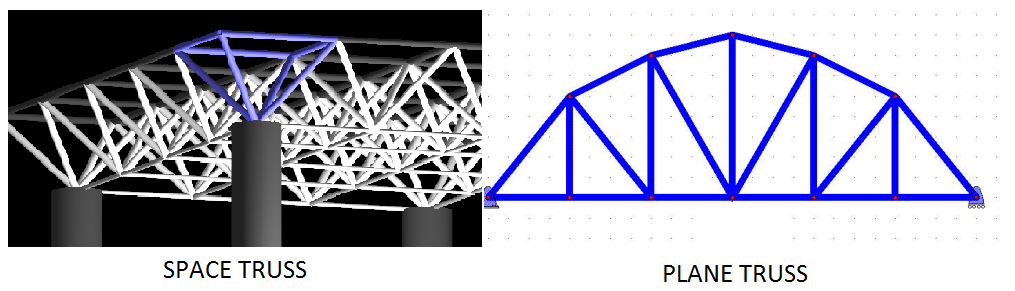
**1. Introduction**

Trusses are widely used in civil and structural engineering to realize very complex systems. Trusses are incorporated in many structures such as bridges, buildings, airplane fuselages, and wings as seen in Figure 1. The term truss was originally derived from the French term *trousse,* which means a *collection of things bound together* [3]. In simplest terms a truss is a two-force member that allows for external load and reactions to act upon the ends of the truss members [1, 2, 4]. A truss system is comprised of a collection of individual truss elements that form the structure [1, 2, 4]. No moment loads are sustained by the truss members, thus the joints at the member ends are assumed to be revolute. This condition is especially important for straight truss members.



**Fig.1**. Visual examples of common truss applications.

All truss systems can be generally categorized into two groups: plane and space trusses. Plane trusses are defined in a two-dimensional plane. This definition is appropriate when all the loads act in the plane either horizontally or vertically, and any out-of-plane reactions are negligible. On the other hand, space trusses are defined in three-dimensional space and are necessary when out of plane loads need to be accounted for [1, 2]. An example of typical space and plane truss configurations can be seen in Figures 2a and 2b respectively.



(a)

(b)

**Fig. 2**. Visualization of a 3-D space truss (a) and 2-D plane truss (b).

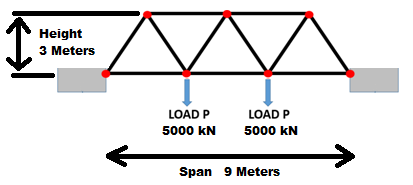
Design optimization principles can be applied to minimize the mass and cost of the structure while maintaining sufficient strength and stiffness. While simple in nature, bridges can be very complex structural systems that require a multi-disciplinary approach to achieve a successful product. In general, bridge design considerations are subject to loading requirements, thermal expansion, vibrations due to wind loading, stiffness, and environmental degradation.

**2. Problem Description**

Following section describes the bridge dimensions chosen for the optimization study along with loading requirements. A brief section will be devoted to outlining the finite element analysis (FEA) method, which is utilized to calculate stresses in the truss members. Next, materials chosen for the analysis are briefly discussed. Finally, the optimization problem is defined and solution methodology is identified.

**2.1 Bridge Definition**

The bridge chosen for the study comprises of truss members formed into equilateral triangles. This type of truss design is more commonly known as a *Warren Truss* [1, 2, 4]. The length of each truss element is set to be 3 meters long. This is a typical truss dimensions found in engineering statics texts [4]. Using the specified dimension yields a bridge of 9 meters in length. This is a relatively short bridge design and is intended to span a small ravine or river. The intent of the proposed bridge is to allow crossing of commercial and private vehicles along with any potential foot traffic. Due to loading and stiffness requirements, the goal is to design a bridge that can hold 5000 kN applied at two locations on the mid-span. A diagram of the proposed bridge, truss elements, and loads can be seen below in Figure 3.



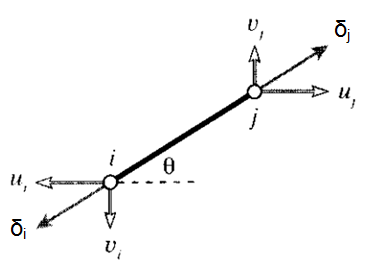
**Fig. 3**. Bridge dimensions

The proposed bridge consists of 11 truss members and 7 joints. Since the applied load acts vertically in the plane, it is appropriate to assume a planar truss analysis. Furthermore, the span is relatively short at 9 meters so any cross-wind loading is insignificant when compared to the vertical loading requirements. As such it is safe to assume that the out of plane loading is negligible.

**2.2 Finite Element Analysis Overview**

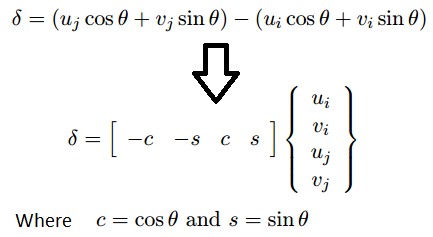
Finite element analysis is widely used across a broad range of engineering fields including but not limited to structural, acoustic, thermal, fluid, material and electrical engineering. In general, if there exist governing partial differential equations (PDE’s) for a particular phenomenon then FEA technique can be applied. For the proposed problem, FEA analysis is applied using truss element formulation to calculate truss member stresses. This is a critical step since the necessary cross-sectional area of each truss element depends on the calculated stresses.

Presented next is a short overview of truss element formulation used for the FEA analysis. Since the problem is of planar truss type, the truss elements considered allow for 2-D displacements. Figure 4 illustrates a typical truss element definition.



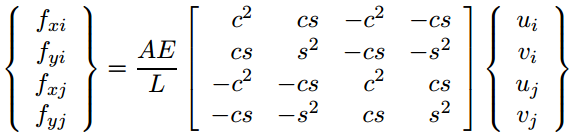
**Fig. 4**. Truss element definition

As seen in Figure 4, the truss element is allowed to displace at nodes *i* and *j*. The node displacements *δi* and δj are along the local element coordinates, requiring one of the basis vectors to be along the element length. The goal is to resolve the nodal displacement in local coordinates to global coordinates. This is done via the rotation matrix as seen in Figure 5. In other words, displacement of the nodes is resolved along directions and, which are defined in global coordinates.



**Fig. 5**. Node displacement defined in global coordinates.

Once the nodal displacements are expressed in global coordinates it is possible to formulate the relationship between nodal displacements and forces as. In this tensor notation, is denoted as the element stiffness matrix and comprises of physical properties that enable a linear mapping between displacement vector and force vector . Explicitly this equation can be expressed as shown in Figure 6 [7, 9].



**Fig. 6**. Truss element force displacement relationship.

Terms A, E, and L correspond to cross-sectional area, Young’s Modulus, and element length respectively. At this stage, it is possible to define the above expression for all the elements in the model (in this case 11 elements). Once each element is defined per equation in Figure 6, each of the 11 expressions is assembled into the global stiffness matrix. When assembling the global stiffness matrix that represents the entire structure, it is important that force equilibrium and displacement compatibilities are maintained. Finally the problem can be solved by inverting the global stiffness matrix to obtain following expression that solves for nodal displacements. Knowing the displacements allows for calculation of elements strains that are used to calculate element stresses via constitutive relationship (i.e. Young’s Modulus, E).

The FEA method was implemented in MATLAB and the subject code can be seen in Appendix A. Before using the FEA solver for optimization, its accuracy was confirmed using commercial FEA software ABAQUS. Model geometry follows the definition in Figure 3 and Table 1 lists the material and geometrical properties used.

|  |  |
| --- | --- |
| **Parameter** | **Model Properties** |
| Young’s Modulus, E | 200 GPa |
| Poisson’s Ratio, ν | 0.30 |
| Cross-Section Area | 0.020 m2 |
| Element Length | 3 m |
| Load, L | 5000 kN |

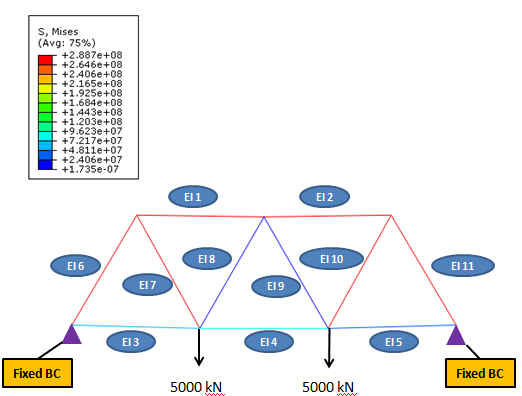
**Table 1**. Model properties.

Steel material properties were obtained from a popular mechanics of materials textbook while geometrical properties were derived from problem definition in Figure 3 [8]. Table 2 lists stress results from the MATLAB solver and ABAQUS. Letter “C” denotes truss elements in compression and “T” identifies elements in tension.

|  |  |  |
| --- | --- | --- |
| **Stress Results for Matlab and ABAQUS analysis** | | |
| **Element #** | **Matlab Stress** | **ABAQUS Stress** |
| 1 | 288.7 MPa, C | 288.7 MPa, C |
| 2 | 288.7 MPa, C | 288.7 MPa, C |
| 3 | 48.1 MPa, C | 48.1 MPa, C |
| 4 | 96.2 MPa, T | 96.2 MPa, T |
| 5 | 48.1 MPa, C | 48.1 MPa, C |
| 6 | 288.7 MPa, C | 288.7 MPa, C |
| 7 | 288.7 MPa, T | 288.7 MPa, T |
| 8 | 0 MPa | 0 MPa |
| 9 | 0 MPa | 0 MPa |
| 10 | 288.7 MPa, T | 288.7 MPa, T |
| 11 | 288.7 MPa, C | 288.7 MPa, C |

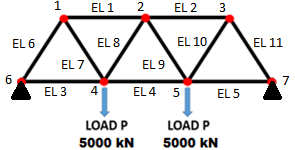
**Table 2**. FEA stress results for MATLAB and ABAQUS solvers.

From the results in Table 2, it was determined that the accuracy of the MATLAB implementation is well within an acceptable margin of error in comparison to the results from ABAQUS. To better visualize the stress results, Figure 7 illustrates node displacements and truss element stresses from ABAQUS as a 2-D plane truss diagram with each element labeled.



**Fig 7**. ABAQUS plot results.

An FEA model for the proposed bridge system is illustrated in Figure 8, which shows model nodes elements and boundary conditions (displacement, load).



**Fig 8**. FEA model for bridge design optimization.

**2.3 Material Selection**

In civil engineering, steel is the overwhelmingly common choice of material used for truss bridge construction [10]. Typically, steel alloy grades can be specified via chemical composition or mechanical strength properties such as yield strength (σy). Yield strength is a material property that defines a stress limit at which permanent deformation occurs [8]. Since yield strength in this study is the only relevant material constraint on the bridge design, following steel grades were chosen as seen in Table 3.

|  |  |  |
| --- | --- | --- |
| **Steel Data** | | |
| **Steel Nomenclature** | **Yield Strength** | **Price per Ton** |
| 270 Steel | 270 MPa | $550.00 |
| 340 Steel | 340 Mpa | $650.00 |
| 420 Steel | 420 Mpa | $700.00 |
| 550 Steel | 550 Mpa | $950.00 |

**Table 3**. Steel properties and costs [5, 6].

The values listed in the table above were obtained through material specifications and current steel prices listed by vendors online [5, 6]. It should be noted that steel prices can vary significantly in short period of time according to fluctuations in supply and demand so quoted prices above may not be accurate now compared to time of this study. Other material properties required are listed in Table 4 [5, 8]. Material properties listed are similar for the same family of steels that were chosen for this project.

|  |  |
| --- | --- |
|  | **Model Properties** |
| Young’s Modulus, E | 200 GPa |
| Poisson’s Ratio, ν | 0.30 |
| Density, ρ | 7850 |

**Table 4.** Additional material properties [5, 8]

**2.4 Optimization Problem Statement**

Cost function is expressed as the minimization of the total mass of the bridge, which is the summation of each individual truss members defined below:

, where

* = Bridge mass, kilograms
* = Cross-section area of each truss (**design variable**), *meters2*
* = Length of each truss element (**constant**), *meters*
* = material density (**constant**),

Element lengths are to remain constant since the size of the bridged ravine is not changing. Therefore, the design variable in the formulation is the cross-sectional areal of each truss element. The structure is required to withstand a load of 5000 kN at two locations on the mid-span as illustrated previously in Figure 3 while requiring that material response remain elastic, ensuring that no permanent deformation is present in the bridge. As such the constraints require that each truss element not exceed yield stress in either compression or tension to avoid permanent deformation. Yield limits can be found in Table 3. Stress constraints are defined below:

, Tensile stress constraint

, Compression stress constraint

* = Element tensile stress constraint
* = Element compressive stress constraint
* = Element stress, Pa
* = Allowable element stress, in this case yield strength, Pa

Since the structure is comprised of 11 truss elements, 22 total stress constraints (2 per element) are considered. Design variable is used in the stress calculation as defined in Figure 6. Additionally, in order to have a feasible design, limits must be imposed on the design variable to ensure that truss elements are not unrealistically thick. Furthermore, negative values are not allowed. This side constraint is simply expressed as:

Finally the problem can be stated in standard form seen below:

**Minimize:**

**Subject to:**

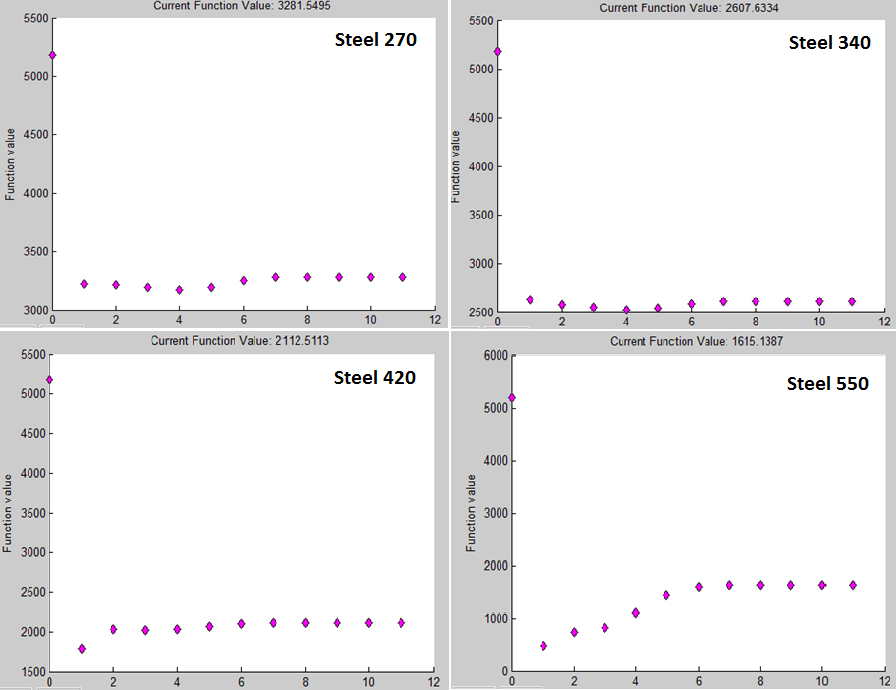
, bounds: [0.0001 – 0.070]m2

The minimum bound for was selected to be a truss of 10x10 mm. This signifies that trusses of smaller dimension are not capable of bearing load. On the opposite end, max truss dimension is set to 265x265 mm. For this design problem, truss thickness greater than 265x265 mm represents the cross-sectional area large enough that is too difficult to handle and connect to neighboring trusses during construction.

While the cost function is linear, the corresponding stress constraints are not. Therefore, MATLAB function FCONMIN was used for the optimization procedure. This method is appropriate for the problem definition above with an acceptable computational cost. No gradient functions were provided at the initial input. Rather, it was decided to allow the FMINCON subroutine to calculate the appropriate gradients using the finite difference procedure.

**3.0 Optimization Results**

To start the optimization procedure, an initial guess was required. The initial chosen was 0.02 m2 for each element. The optimization subroutine was executed for each of the materials listed in Table 3. The iteration history for the four runs is summarized in Figure 9. As seen in Figure 9, each optimization run converged in 11 iterations in a smooth manner with no zig-zag patterns visible. This implies good convergence performance with respect to the defined constraints.



**Fig 9**. Optimization iteration history

Next, Table 5 shows the optimized values from the FEA solver. It can be observed that as steel yield strength is increased, optimized cross-section areas decrease in value. This result is expected since stronger materials can withstand higher stresses before plasticity occurs.

|  |  |  |  |  |  |
| --- | --- | --- | --- | --- | --- |
| **Element ID** | **Initial Guess (m2)** | **Steel 270 (m2)** | **Steel 340 (m2)** | **Steel 420 (m2)** | **Steel 550 (m2)** |
| 1 | 0.020 | 0.021 | 0.017 | 0.014 | 0.011 |
| 2 | 0.020 | 0.021 | 0.017 | 0.014 | 0.011 |
| 3 | 0.020 | 0.0001 | 0.0001 | 0.0001 | 0.0001 |
| 4 | 0.020 | 0.011 | 0.008 | 0.007 | 0.005 |
| 5 | 0.020 | 0.0001 | 0.0001 | 0.0001 | 0.0001 |
| 6 | 0.020 | 0.021 | 0.017 | 0.014 | 0.011 |
| 7 | 0.020 | 0.021 | 0.017 | 0.014 | 0.011 |
| 8 | 0.020 | 0.0001 | 0.0001 | 0.0001 | 0.0001 |
| 9 | 0.020 | 0.0001 | 0.0001 | 0.0001 | 0.0001 |
| 10 | 0.020 | 0.021 | 0.017 | 0.014 | 0.011 |
| 11 | 0.020 | 0.021 | 0.017 | 0.014 | 0.011 |

**Table 5**. Optimized values for for 4 steel variants.

Each of the material columns has only 3 distinct area values. This is attributed to the geometric and loading symmetry present in the bridge design. It is also important to note that while the optimized result indicates elements 3, 5, 8, and 9 have very small *A*, they cannot be removed entirely from the bridge design since undesirable asymmetries and instabilities are introduced. These elements can however be made very thin. Finally, it can be observed that optimized cross-section areas are within the defined bounds of the optimization problem. The next variable of interest is the resultant stresses in the truss members, which can be viewed in Table 6.

|  |  |  |  |  |  |
| --- | --- | --- | --- | --- | --- |
| **Truss Elements Stresses expressed as 1.0\*e8 Pa** | | | | | |
| **Element ID** | **Initial Stress** | **Steel 270** | **Steel 340** | **Steel 420** | **Steel 550** |
| 1 | -2.887 | -2.700 | -3.400 | -4.200 | -5.500 |
| 2 | -2.887 | -2.700 | -3.400 | -4.200 | -5.500 |
| 3 | -0.481 | -1.350 | -1.700 | -2.100 | -2.750 |
| 4 | 0.962 | 2.700 | 3.400 | 4.200 | 5.500 |
| 5 | -0.481 | -1.350 | -1.700 | -2.100 | -2.750 |
| 6 | -2.887 | -2.700 | -3.400 | -4.200 | -5.500 |
| 7 | 2.887 | 2.700 | 3.400 | 4.200 | 5.500 |
| 8 | 0.000 | 0.000 | 0.000 | 0.000 | 0.000 |
| 9 | 0.000 | 0.000 | 0.000 | 0.000 | 0.000 |
| 10 | 2.887 | 2.700 | 3.400 | 4.200 | 5.500 |
| 11 | -2.887 | -2.700 | -3.400 | -4.200 | -5.500 |

**Table 6**. Stress results from the optimization output.

Results in Table 6 show that as constraint limits are increased for higher strength steel variants, the optimizer succeeds at reaching the constraint boundary for a selected group of elements (elements 1, 2, 4, 6, 7, 10, 11). In fact the stress constraints are active for the aforementioned elements. Stress results shown in Table 6 also support results in Table 5, which show that a very small stress response in a truss element can allow a near-zero cross-sectional area (elements 3, 5, 8, 9). It should be noted that negative signs in Table 6 denote a truss element in compression.

Table 7 contains constraint values, which serve to confirm that the achieved optimum design is feasible. Constraint values are tabulated for tension and compression. The goal is to have the constraint value be ≤ 0. Any positive value signifies a violated constraint. Out of the four steel types, only steel 270 starting design violated the constraint requirements as indicated by the positive values. The remaining steel grades had no starting design constraint violations.

|  |  |  |  |  |  |  |  |  |
| --- | --- | --- | --- | --- | --- | --- | --- | --- |
| **Stress Constraint Values** | | | | | | | | |
|  | **Steel 270 Const. Values** | | **Steel 340 Const. Values** | | **Steel 420 Const. Values** | | **Steel 550 Const. Values** | |
|  | **Initial** | **Final** | **Initial** | **Final** | **Initial** | **Final** | **Initial** | **Final** |
| **Tensile** | -2.07 | -2.00 | -1.85 | -2.00 | -1.69 | -2.00 | -1.52 | -2.00 |
| -2.07 | -2.00 | -1.85 | -2.00 | -1.69 | -2.00 | -1.52 | -2.00 |
| -1.18 | -1.50 | -1.14 | -1.50 | -1.11 | -1.50 | -1.09 | -1.50 |
| -0.64 | 0.00 | -0.72 | 0.00 | -0.77 | 0.00 | -0.83 | 0.00 |
| -1.18 | -1.50 | -1.14 | -1.50 | -1.11 | -1.50 | -1.09 | -1.50 |
| -2.07 | -2.00 | -1.85 | -2.00 | -1.69 | -2.00 | -1.52 | -2.00 |
| 0.07 | 0.00 | -0.15 | 0.00 | -0.31 | 0.00 | -0.48 | 0.00 |
| -1.00 | -1.00 | -1.00 | -1.00 | -1.00 | -1.00 | -1.00 | -1.00 |
| -1.00 | -1.00 | -1.00 | -1.00 | -1.00 | -1.00 | -1.00 | -1.00 |
| 0.07 | 0.00 | -0.15 | 0.00 | -0.31 | 0.00 | -0.48 | 0.00 |
| -2.07 | -2.00 | -1.85 | -2.00 | -1.69 | -2.00 | -1.52 | -2.00 |
| **Compression** | 0.07 | 0.00 | -0.15 | 0.00 | -0.31 | 0.00 | -0.48 | 0.00 |
| 0.07 | 0.00 | -0.15 | 0.00 | -0.31 | 0.00 | -0.48 | 0.00 |
| -0.82 | -0.50 | -0.86 | -0.50 | -0.89 | -0.50 | -0.91 | -0.50 |
| -1.36 | -2.00 | -1.28 | -2.00 | -1.23 | -2.00 | -1.18 | -2.00 |
| -0.82 | -0.50 | -0.86 | -0.50 | -0.89 | -0.50 | -0.91 | -0.50 |
| 0.07 | 0.00 | -0.15 | 0.00 | -0.31 | 0.00 | -0.48 | 0.00 |
| -2.07 | -2.00 | -1.85 | -2.00 | -1.69 | -2.00 | -1.52 | -2.00 |
| -1.00 | -1.00 | -1.00 | -1.00 | -1.00 | -1.00 | -1.00 | -1.00 |
| -1.00 | -1.00 | -1.00 | -1.00 | -1.00 | -1.00 | -1.00 | -1.00 |
| -2.07 | -2.00 | -1.85 | -2.00 | -1.69 | -2.00 | -1.52 | -2.00 |
| 0.07 | 0.00 | -0.15 | 0.00 | -0.31 | 0.00 | -0.48 | 0.00 |

**Table 7**. Constraint values

No constraints are violated at the final design for all steel grades. Note that values of zero indicate active constraints.

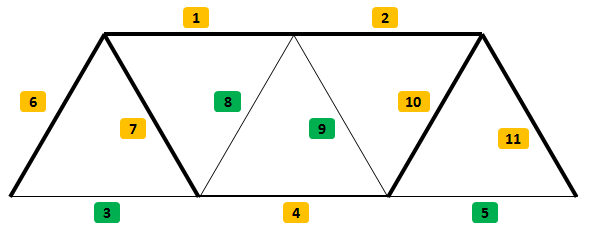
Knowing that the design optimization results are feasible, cost values for the specified steel grades were used in conjunction with final mass values to calculate bridge costs. These results are tabulated in Table 8.

|  |  |  |  |
| --- | --- | --- | --- |
| **Optimization Summary** | | | |
| **Alloy Grade** | **Starting Mass (kg)** | **Ending Mass (kg)** | **Cost ($)** |
| 270 | 5181.0 | 3281.6 | 1805.00 |
| 340 | 5181.0 | 2607.6 | 1695.00 |
| 420 | 5181.0 | 2112.5 | 1479.00 |
| 550 | 5181.0 | 1615.0 | 1534.00 |

**Table 8**. Mass and cost results.

Final results show that steel grade 550 yields the lightest design. This result is expected due to the problem formulation statement in terms of bridge mass; however, a steel grade 550 bridge did not yield the most cost-efficient option. Steel 420 as a building material, while being heavier, yields the cheapest design option at $1479.00. This is a result not immediately discernable without additional cost analysis. Steel grades 340 and 270 produce the costliest design due to the mass penalty incurred in order to maintain elastic behavior.

A visual depiction of the resulting truss element cross-sectional area for the chosen steel 420 design is shown in Figure 10. Elements colored in orange indicate truss elements with active constraints. Elements colored in green have inactive stress constraints.



**Fig. 10**. Visualization of optimized truss element cross-sectional area and depiction of active and inactive constraints for the chosen steel 420 bridge.

As seen in Figure 10, those truss elements with active stress constraints tend to be thicker in cross-sectional area than their inactive stress constraint counterparts.

**4.0 Post-optimality Analysis**

In the earlier section it was shown that the constraint conditions were met and the optimal design is within the feasible region. This section will briefly discuss post-optimality analysis of the optimized results. For the present analysis, Lagrange multipliers are utilized to study the sensitivity of the design with respect to the constraint limits. Subroutine FMINCON has the capability of numerically approximating the Lagrange multipliers, which was utilized here. Table 9 lists the truss element Lagrange multipliers for all the material selections studied.

|  |  |  |  |  |
| --- | --- | --- | --- | --- |
| **Lagrange Multipliers** | | | | |
| **Element ID** | **Steel 270** | **Steel 340** | **Steel 420** | **Steel 550** |
| 1 | 503.58 | 399.91 | 323.74 | 247.20 |
| 2 | 503.58 | 399.91 | 323.74 | 247.20 |
| 3 | 0.00 | 0.00 | 0.00 | 0.00 |
| 4 | 251.80 | 199.96 | 161.87 | 123.60 |
| 5 | 0.00 | 0.00 | 0.00 | 0.00 |
| 6 | 503.58 | 399.91 | 323.73 | 247.20 |
| 7 | 503.58 | 399.91 | 323.73 | 247.20 |
| 8 | 0.00 | 0.00 | 0.00 | 0.00 |
| 9 | 0.00 | 0.00 | 0.00 | 0.00 |
| 10 | 503.58 | 399.91 | 323.73 | 247.20 |
| 11 | 503.58 | 399.91 | 323.73 | 247.20 |

Table 9: Truss element Lagrange multipliers for all material cases.

Table 9 shows that Lagrange multipliers exist only for elements with active constraints. This makes sense because inactive constraint elements have enough slack that prevent any change to the cost function with respect to these truss elements. It is also noteworthy to mention that the Lagrange multipliers exhibit same symmetry as the stress and cross-section area results. Furthermore, Table 9 shows a decrease in Lagrange multiplier values with respect to increase in material yield strength. This implies that the bridge mass is less sensitive to constraint changes as material strength is increased.

Results from Table 9 are used to study the impact to bridge mass and cost if the constraint limits are relaxed. Steel 420 is used for the sensitivity analysis although similar procedure can be carried out for the remaining steel variants. Goal is to calculate the impact to bridge mass and cost if the material strength can be increased by 25 and 50 MPa with respect to the baseline yield strength of 420 MPa. Change to the cost function (bridge mass) can be calculated by the following equation [11].

* = Change in cost function
* = Lagrange multiplier
* = Constraint variation

Using the appropriate Lagrange multipliers along with the expression above, mass and cost impact is calculated for the optimum design using steel 420. Results are tabulated in Table 10. For comparison, optimal mass and cost for steel 420 were also tabulated in the first row.

|  |  |  |  |
| --- | --- | --- | --- |
| **Sensitivity analysis for steel 420** | | | |
| Mpa | (kg) | *Mass (kg)* | *Cost ($)* |
| 0 | 0 | 2112.5 | 1479 |
| 25 | -125.3 | 1987.2 | 1391.1 |
| 50 | -250.5 | 1862 | 1303.4 |

Table 10: Sensitivity study for steel 420 bridge design.

Hypothetically, if steel variants 445 and 470 were available on the market, sensitivity analysis in Table 10 can be used to determine the impact to the bridge mass and cost. Cost for the new designs were calculated assuming steel 420 pricing.

**5.0 Optimization Summary**

Optimization analysis of a conventional truss bridge was carried out. Stress in the truss members was calculated via FEA technique that was employed inside MATLAB. Stress results obtained from MATLAB were confirmed by ABAQUS CAE software, which is a commercial package widely used in academia and industry.

An FEA solver was used in conjunction with MATLAB function FMINCON to optimize a truss bridge design with 11 truss members as seen in Figure 10 with two 5000 kN loads as shown in Figure 3. Four steel grades were chosen for the study, each of which had its respective yield strength value. Stress constraints employed in the optimization analysis utilized individual yield strength values to define feasible design regions. Each optimization simulation required 11 iterations to converge, which was smooth in nature. Due to geometrical and loading symmetries, optimization results were also symmetric. Final optimization results have active stress constraints for certain group of elements (elements 1, 2, 4, 6, 7, 10, 11). This was true for all steel grades. Additionally, the optimized results identified truss elements with low stresses (elements 3, 5, 8, 9). In these cases, their respective cross-section area was reduced to the lower limit of the side constraint. Low stress truss elements should remain in the bridge structure in order to ensure structural stability. However, their respective cross-section area footprint should be very small. Constraint values for all the design options were ≤ 0, which implies that optimization results remained in feasible space.

Optimized bridge mass for all steel grades were used to calculate total material costs. While steel grade 550 achieved lightest design and therefore least steel material usage, due to material costs it was not the most cost efficient. A steel grade 420 bridge, while heavier than a 550 steel bridge, had the lowest material costs at $1479.00 vs $1534.00 respectively. This result is not immediately recognizable without performing additional material cost analysis. Steel grades 270 and 340 were the cheapest options in terms of material cost; however, their resultant bridge mass to compensate for element stresses was enough to make these options the costliest in terms of total monetary costs. Finally, sensitivity analysis was carried out on steel 420 optimization results. It was determined that if yield strength is increased by 25 and 50 MPa respectively the corresponding weight reduction of the truss bridge is 125.3 and 250.5 kg. Sensitivity analysis also determined that bridge design is more sensitive when weaker steel variant are used.

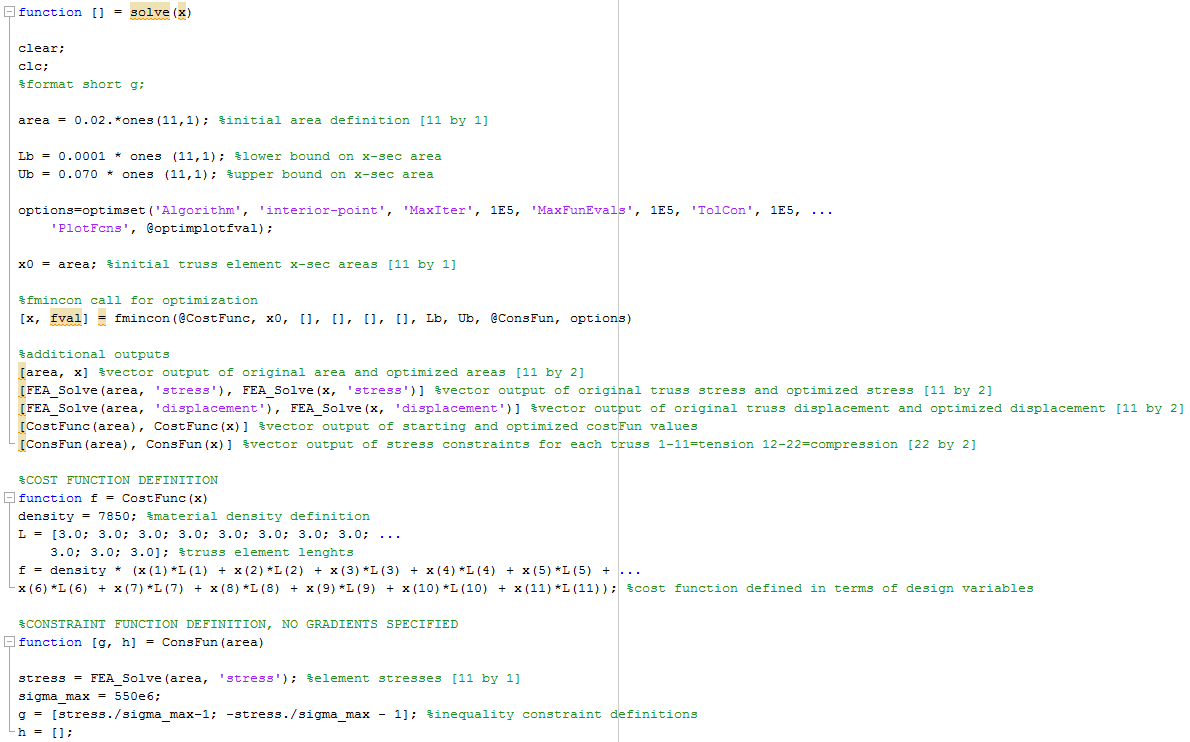
In closing, a truss bridge optimization procedure was carried out with the aid of an FEA solver in MATLAB. The project goal was to obtain the cheapest bridge design, which was achieved successfully.

**References**

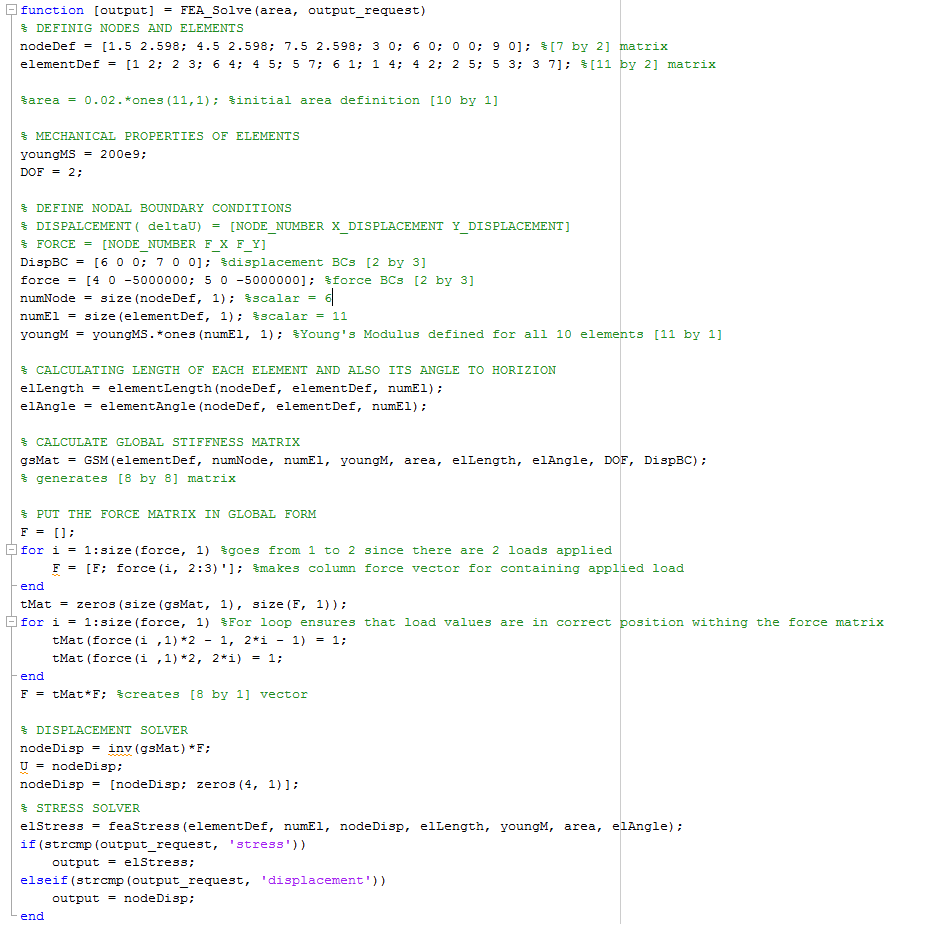
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**Appendix A**

**M-file to initiate optimization procedure:**



**FEA Solver:**

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**Additional Support Files:**

